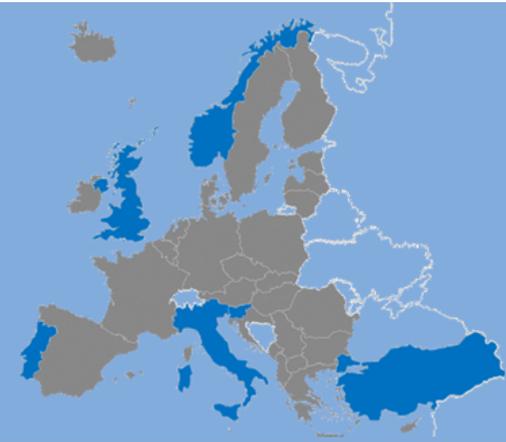




European
Commission

Horizon 2020
European Union funding
for Research & Innovation

liquefACT



EARTHQUAKE INDUCED LIQUEFACTION RISK: HOLISTIC ASSESSMENT AND MITIGATION

Wednesday 20th June 2018 - 11:30-13:00

ROOM: CR2 (building M2 - Thessaloniki Concert Hall/16ECEC
Conference Venue)



Empirical damage and liquefaction fragility curves

Andrea Prota, Alessandro Flora, Marco Di Ludovico, Emilio Bilotta,
Anna Chiaradonna, Pietro Caputo

UNIVERSITY OF NAPOLI FEDERICO II



INTRODUCTION

- DATA COLLECTION ON LIQUEFACTION INDUCED DAMAGE**



Agenzia regionale per la Ricostruzione Sisma 2012

2017 - Scientific agreement



UNIVERSITÀ DEGLI STUDI DI NAPOLI
FEDERICO II

DATA COLLECTION ON DAMAGED BUILDINGS

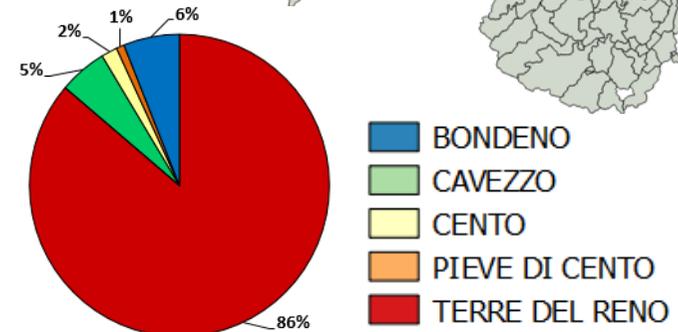
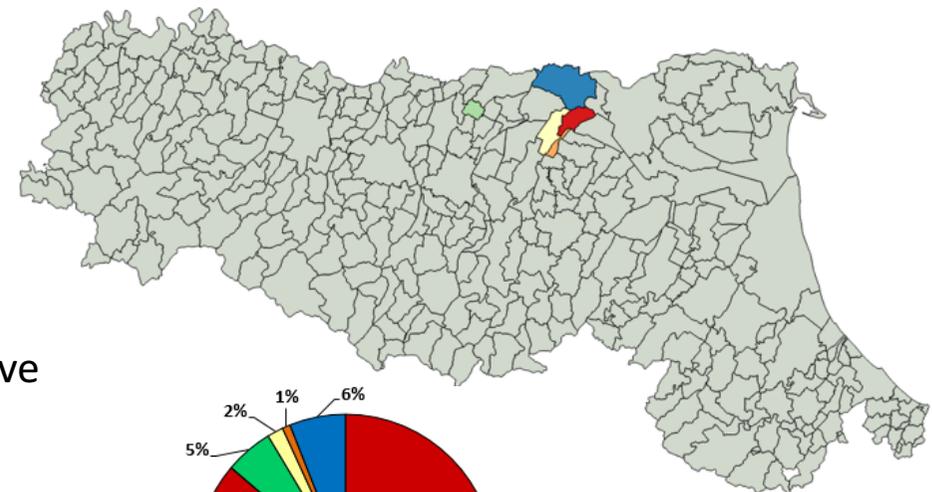
(Municipality of Mirabello, San Carlo (Sant'Agostino), Bondeno, Cavezzo, Cento, and Pieve di Cento)

DATA BY USABILITY SURVEY FORMS AND PRACTITIONERS' REPORT

(Level and extent of damage on structural and non structural members, photo reports of damage, destructive and non destructive tests on materials and ground, etc.)

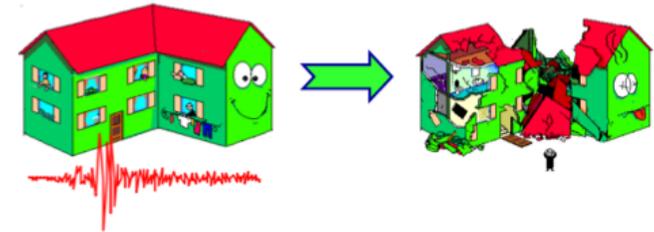
Liquefied Ground Buildings (LG): 116 masonry buildings
with public grant for liquefaction induced damage

Unliquefied Ground Buildings (UG): 993 masonry buildings
with public grant for seismic inertial forces induced damage



OBJECTIVES

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE:
 Can we clearly distinguish liquefaction induced damage from seismic one?

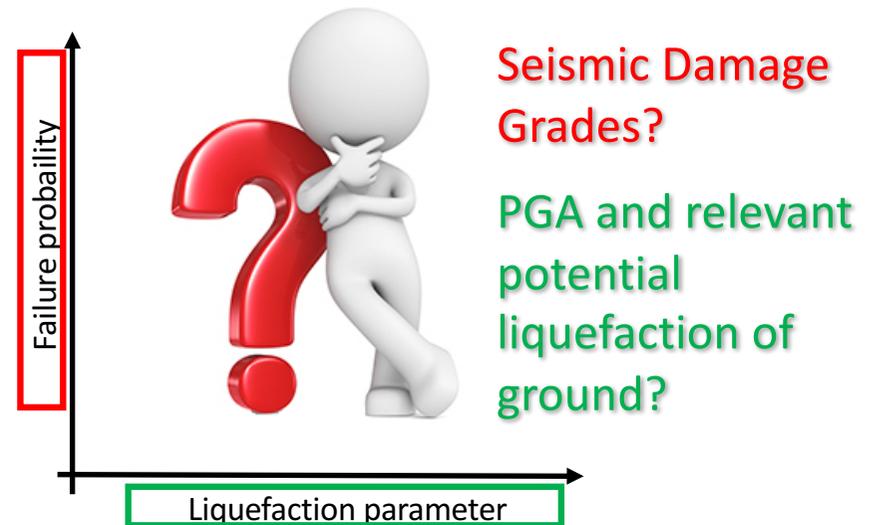


Level - extension Structural component Pre-existing damage	DAMAGE									
	D4-D5 Very heavy			D2-D3 Medium-severe			D1 Slight			Null
	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	
A	B	C	D	E	F	G	H	I	L	
1 Vertical structures	<input type="checkbox"/>									
2 Floors	<input type="checkbox"/>									
3 Stairs	<input type="checkbox"/>									
4 Roof	<input type="checkbox"/>									
5 Infills-partitions	<input type="checkbox"/>									
6 Pre-existing damage	<input type="checkbox"/>									

The AeDES form (Baggio et al. 2008)

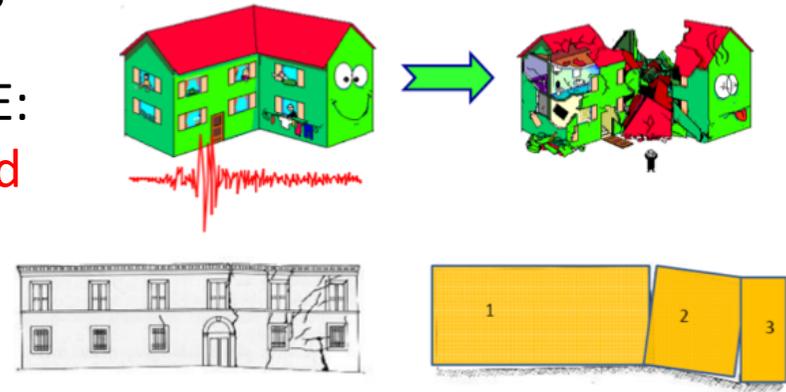
II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE:
 How much the global building damage is affected by liquefaction?

III. LIQUEFACTION FRAGILITY CURVES:
 Which parameters should be used and what is the fragility curve trend?



OBJECTIVES

- I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE:
Can we clearly distinguish liquefaction induced damage from seismic one?



DAMAGE ANALYSIS – CRACK PATTERN

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Summary table report



Scuola Politecnica e delle Scienze di Base - Università degli Studi di Napoli Federico II

Pratica 13











FOR EACH BUILDING:

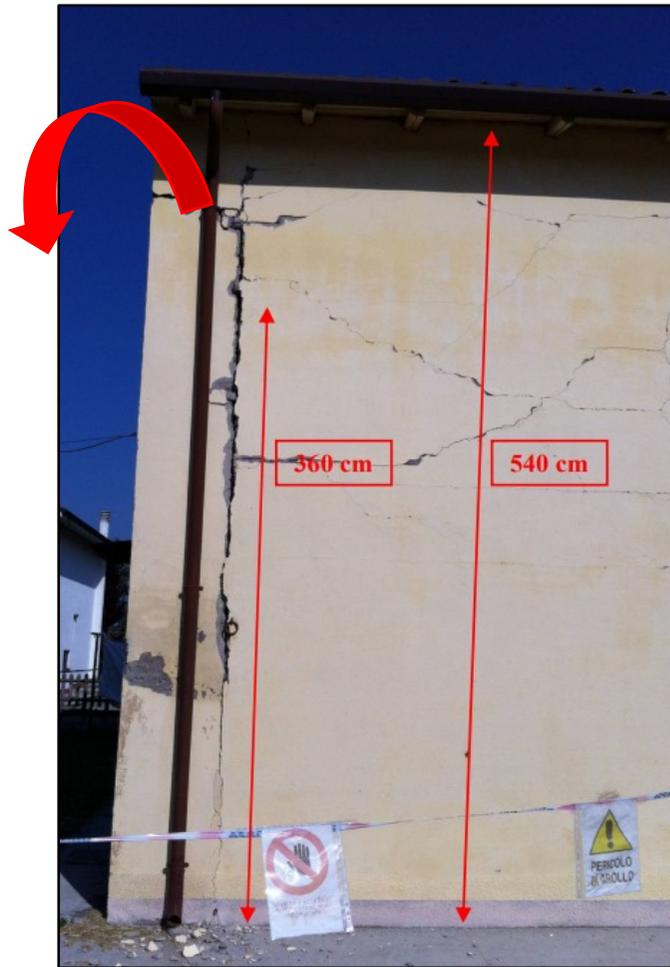
- Plan, elevation and section view;
- Damage photos;
- Damage localization;
- Analysis of damage

DAMAGE ANALYSIS – CRACK PATTERN

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Damage - Seismic Inertial actions

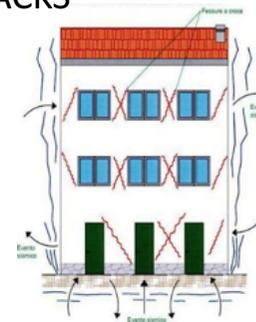
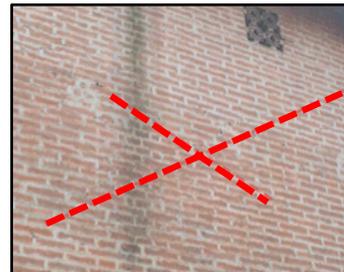
WALL SIMPLE OVERTURNING



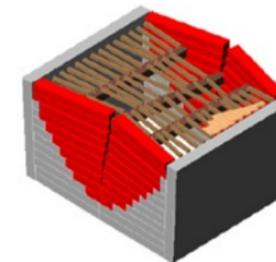
ROOF OVERTURNING



DIAGONAL CRACKS



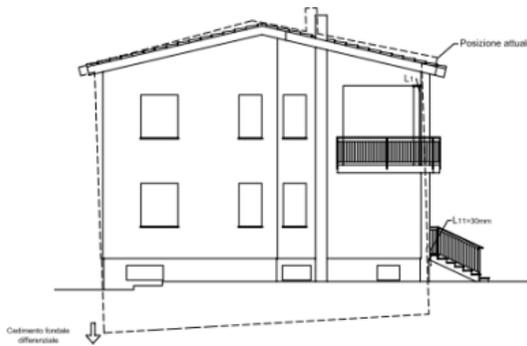
FLOOR WALL CONNECTION



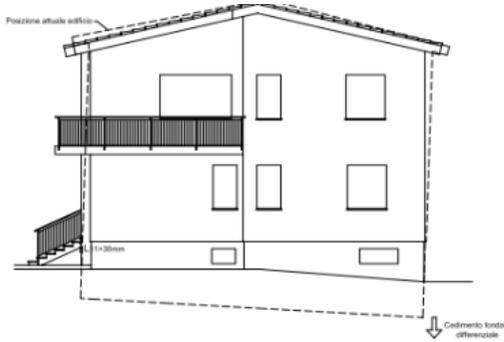
DAMAGE ANALYSIS – CRACK PATTERN

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

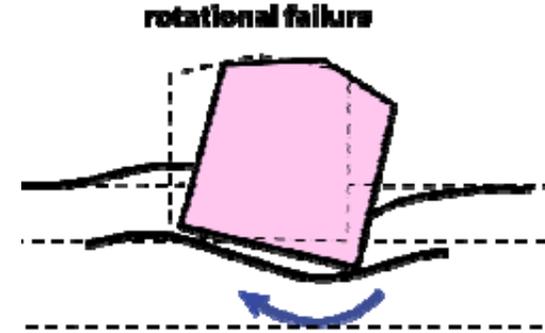
Damage - Liquefaction



Prospetto Ovest



Prospetto Est



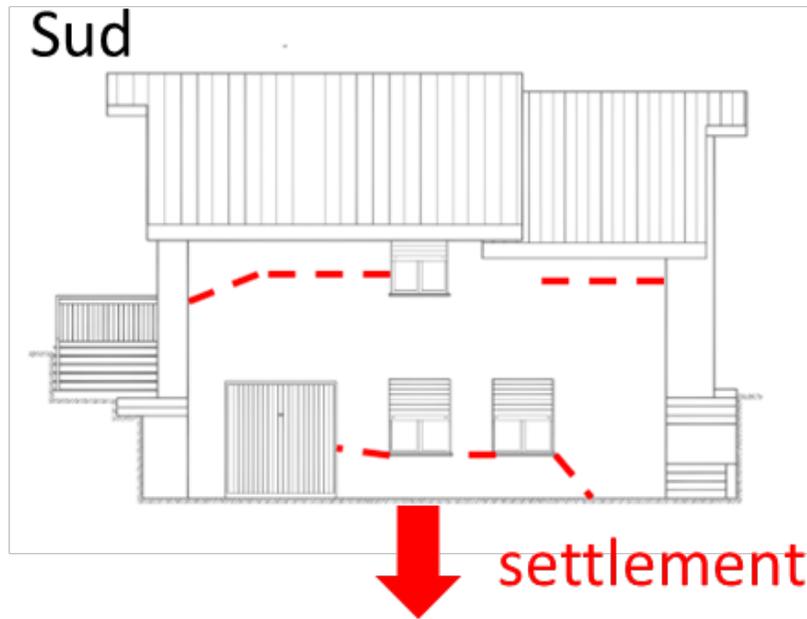
SETTLEMENTS- RIGID ROTATION OF BUILDING



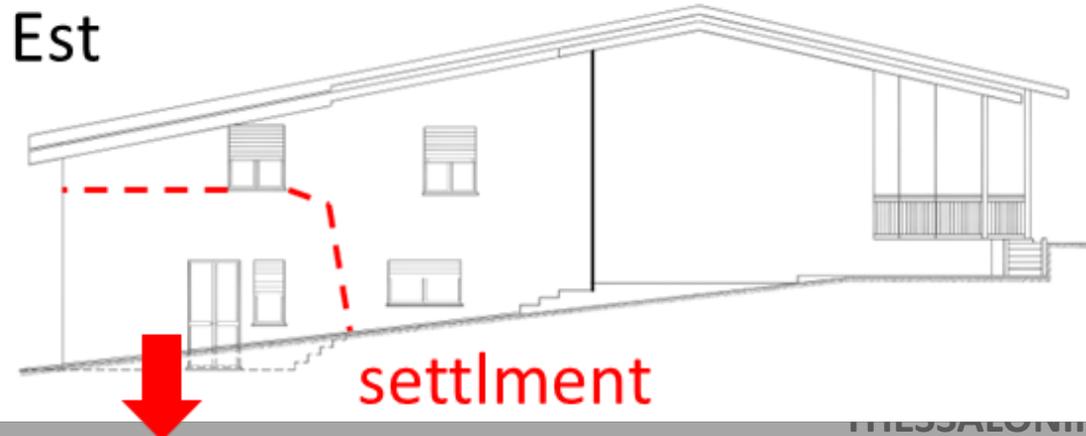
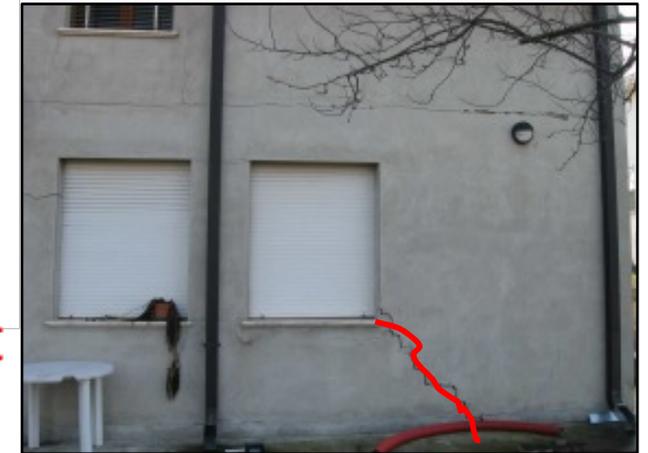
DAMAGE ANALYSIS – CRACK PATTERN

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Damage - Liquefaction



LOCALIZED SETTLEMENTS



DAMAGE ANALYSIS – CRACK PATTERN

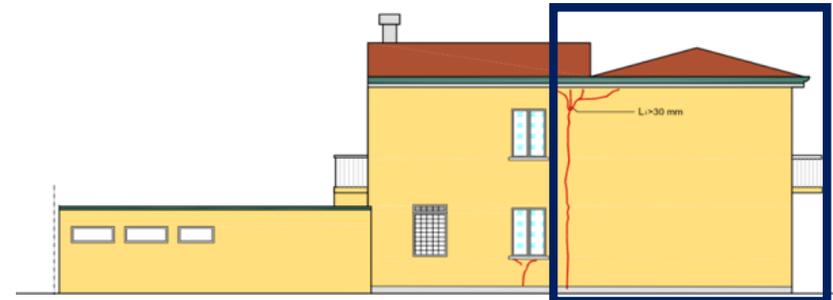
I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Damage - Liquefaction



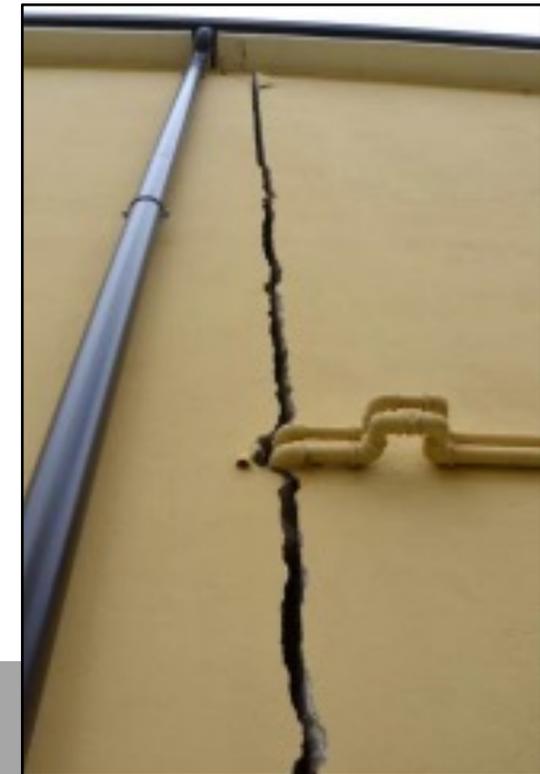
Prospetto nord-est

Lateral Displacement



Prospetto sud

Lateral Displacement



DAMAGE ANALYSIS – CRACK PATTERN

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Damage - Liquefaction



settlements



LOCALIZED SETTLEMENTS



DAMAGE ANALYSIS – CRACK PATTERN

I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Damage – Liquef. + Inertial forces



PROSPETTO SUD

...in several cases both typical damage induced by inertial forces and liquefaction (settlement) is detected on buildings

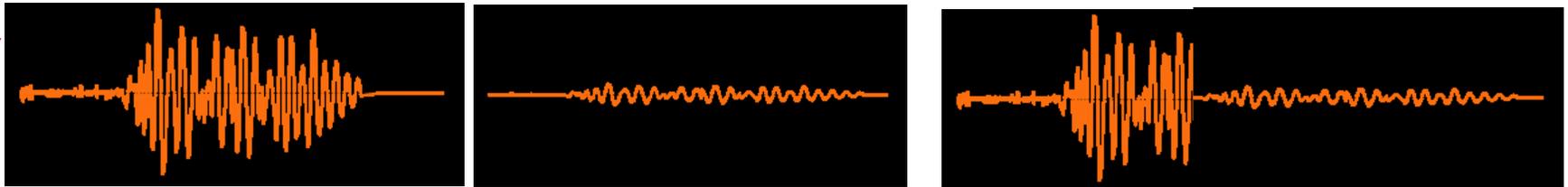
...the liquefaction is a «filter» to seismic actions on superstructure but it can be not immediate

DAMAGE ANALYSIS – CRACK PATTERN

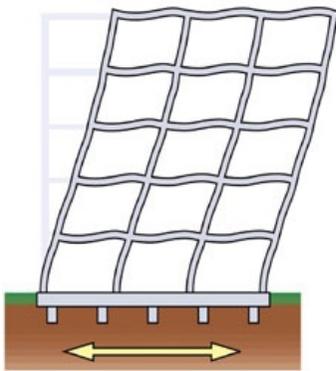
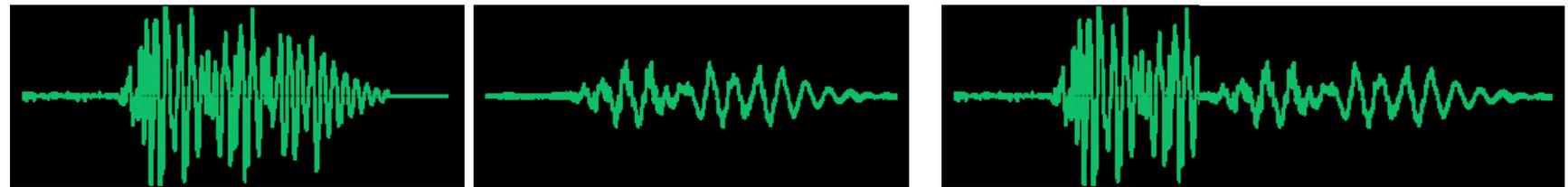
I. ANALYSIS OF LIQUEFCTION INDUCED DAMAGE

Damage – Liquef. + Inertial forces

Interstorey Drift



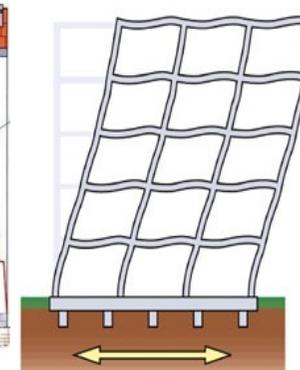
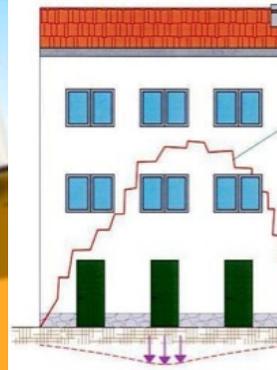
Structural response (acceler.)



No Liquefaction



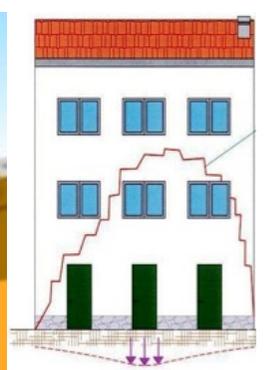
Liquef. activation



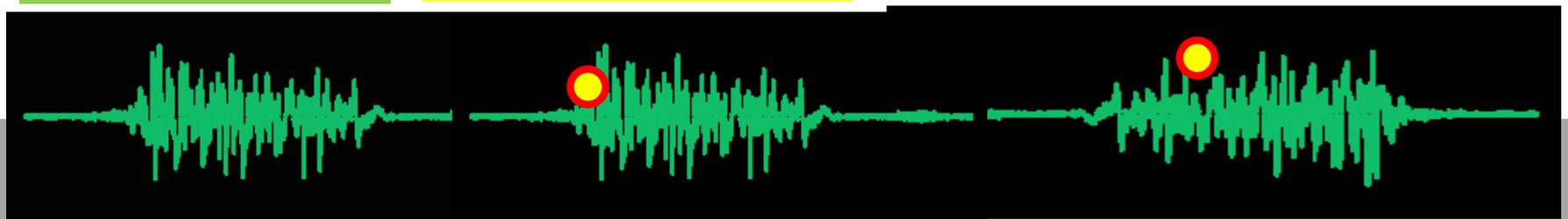
No Liquef.



+Liquefaction



Acceler. Input



OBJECTIVES

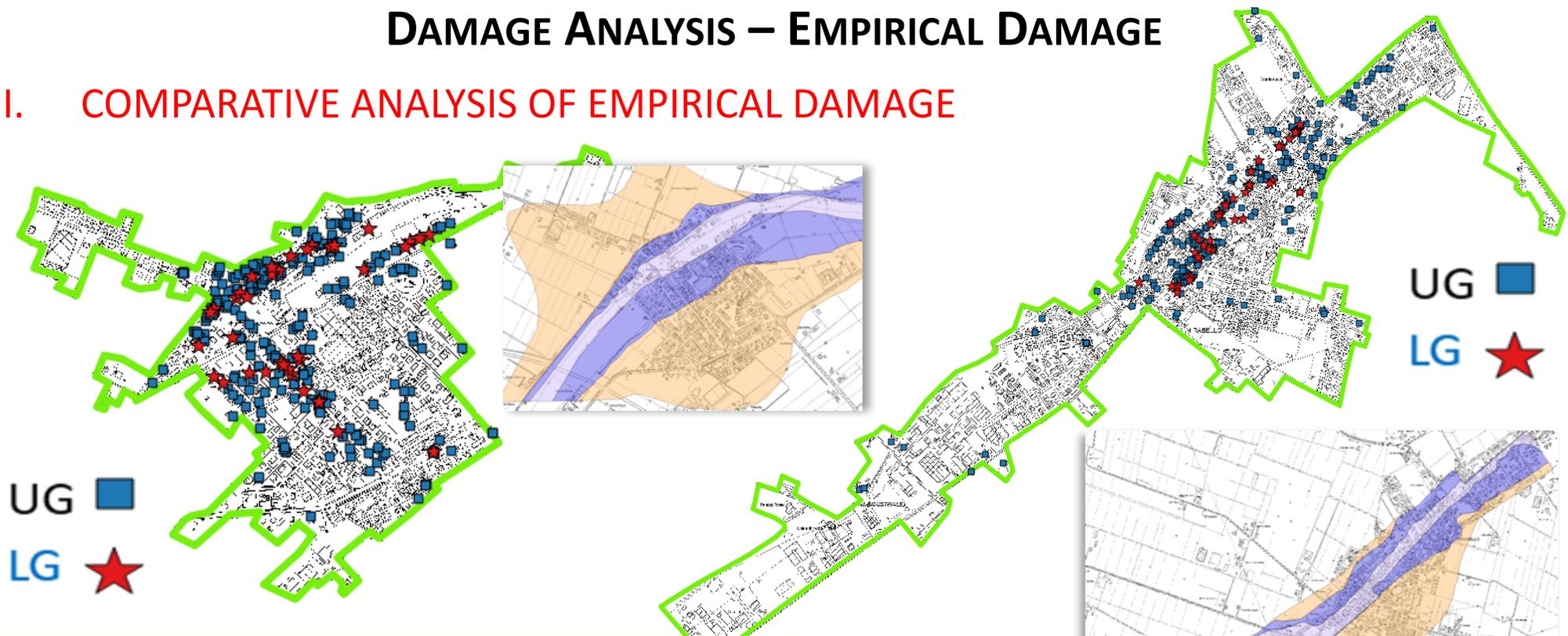
Level - extension Structural component Pre-existing damage	DAMAGE										
	D4-D5 Very heavy			D2-D3 Medium-severe			D1 Slight			Null	
	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3		
	A	B	C	D	E	F	G	H	I		L
1 Vertical structures	<input type="checkbox"/>										
2 Floors	<input type="checkbox"/>										
3 Stairs	<input type="checkbox"/>										
4 Roof	<input type="checkbox"/>										
5 Infills-partitions	<input type="checkbox"/>										
6 Pre-existing damage	<input type="checkbox"/>										

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE:
 How much the global building damage is affected by liquefaction?

The AeDES form (Baggio et al. 2008)

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

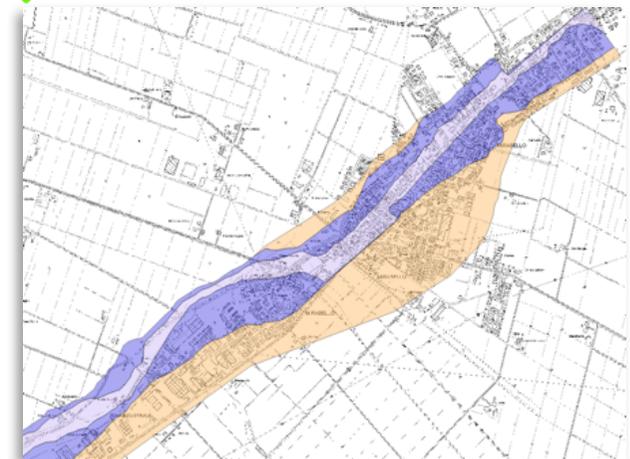
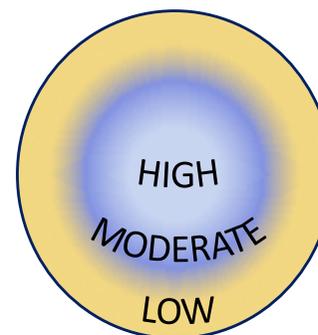


Empirical damage information on 1094 buildings

	SAN CARLO	MIRABELLO	TOT.
LG BUILD.	46	55	101
UG BUILD.	393	600	993

According to census data (ISTAT) there are 439+675=1114 masonry buildings in San Carlo and Mirabello (no damage assumed on unsurveyed buildings, 363 build.)

POTENTIAL LIQUEFACTION



- PLAIN
- PALEO-BANK
- PALEO-RIVERBED

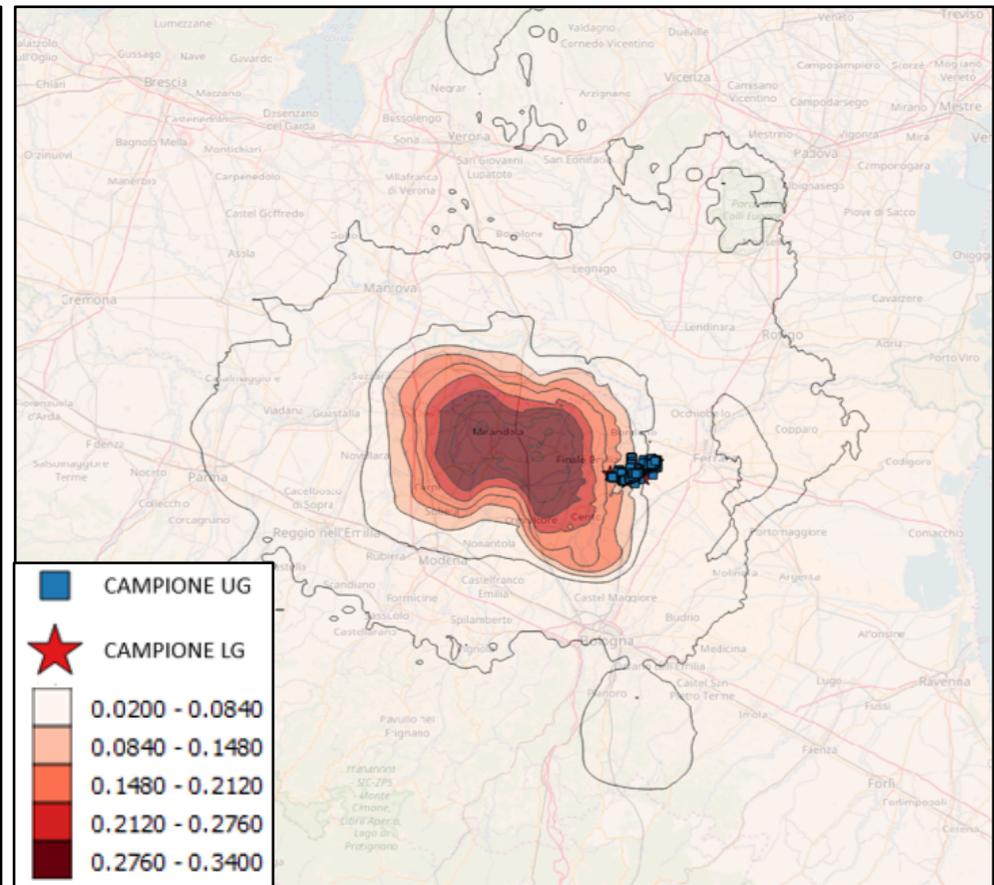
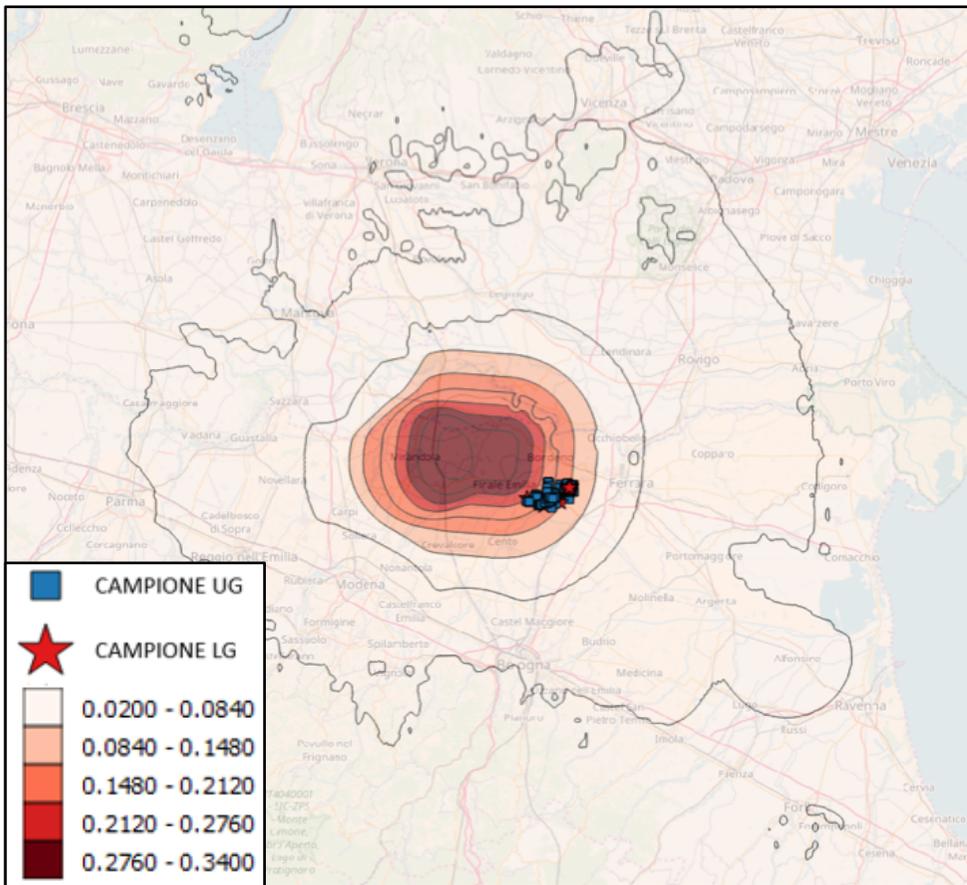
DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: How much set of buildings are comparable?

INGV ShakeMap: 20 May 2012 02:03:50 UTC M 5.9

INGV ShakeMap: 29 May 2012 07:00:03 UTC M 5.8

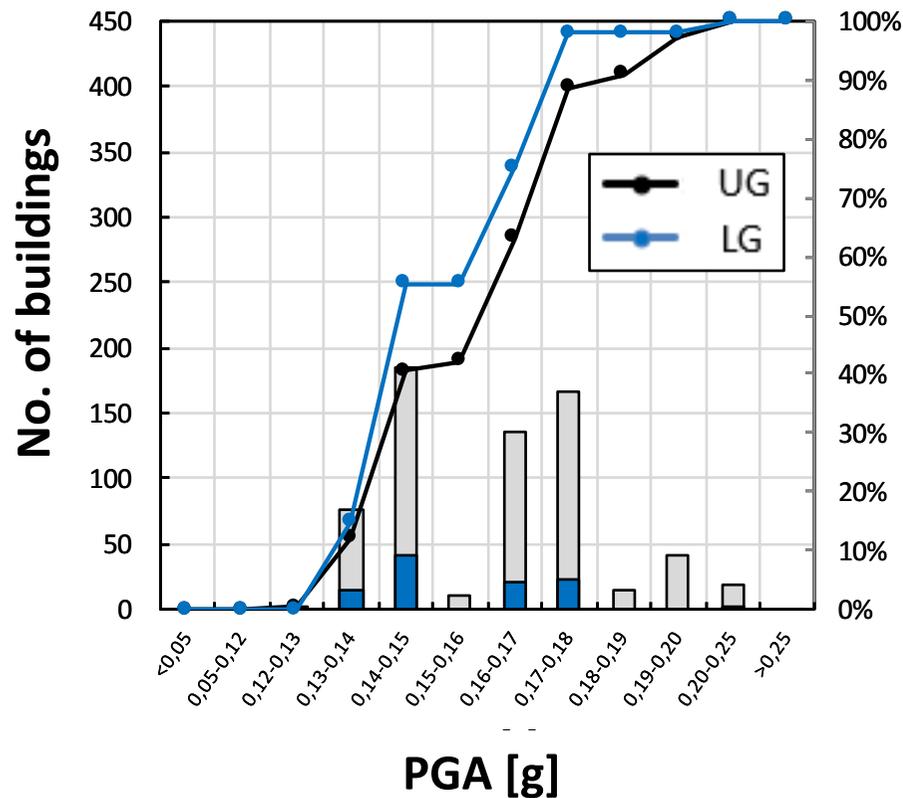


DAMAGE ANALYSIS – EMPIRICAL DAMAGE

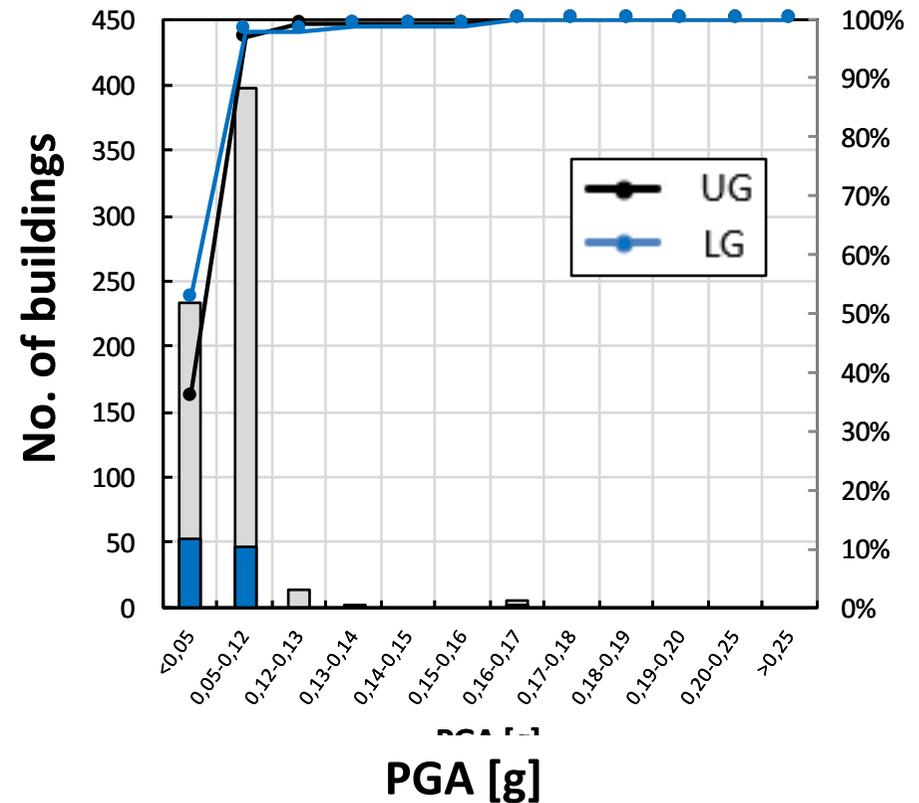
II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: How much set of buildings are comparable?

❖ PGA MAY 20th



❖ PGA MAY 29th



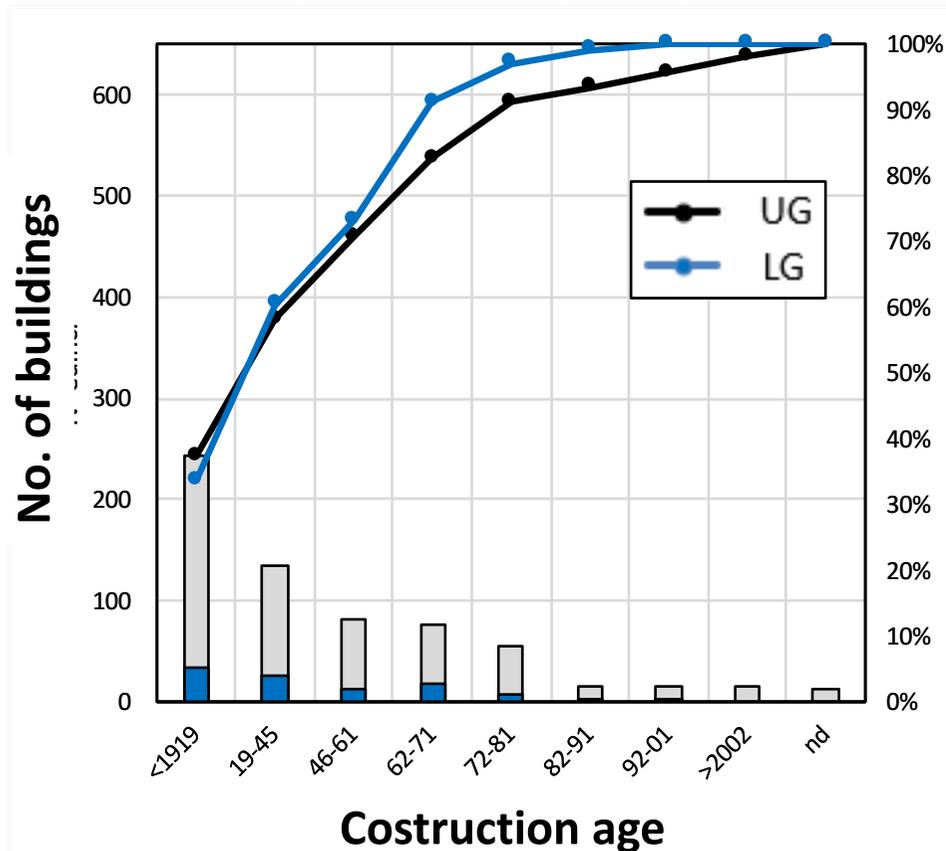
..... similar PGA values mainly within the range 0.13 -0.18 g

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

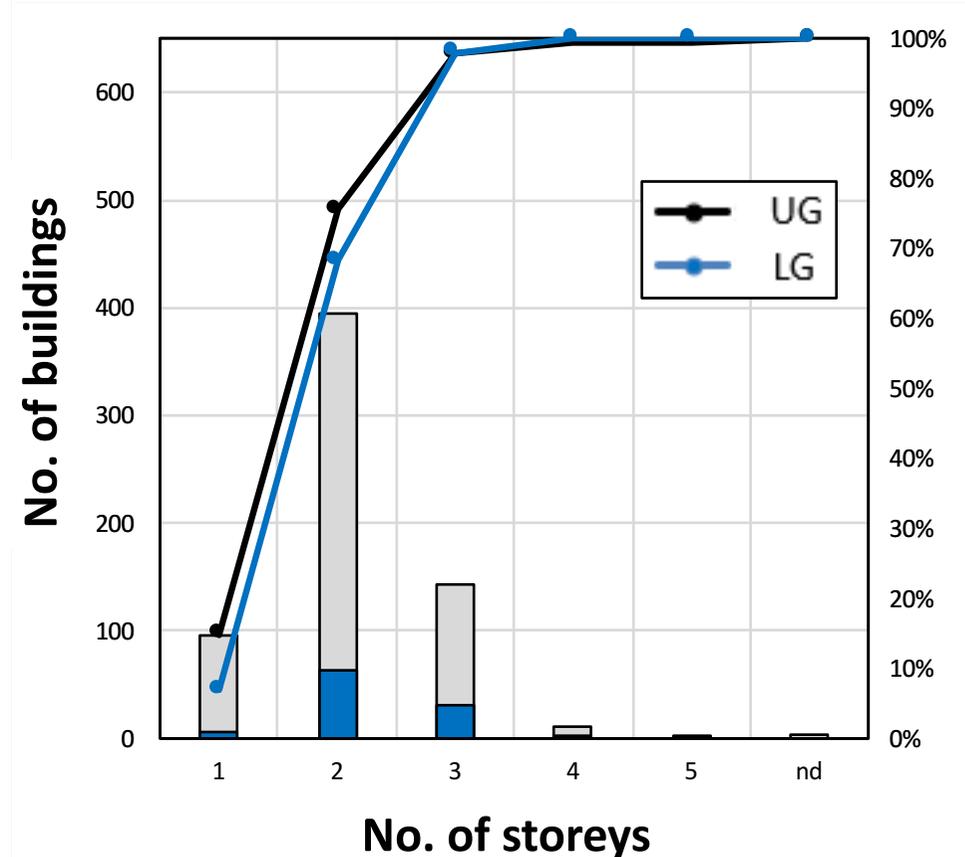
II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: How much set of buildings are comparable?

❖ CONSTRUCTION AGE



❖ NUMBER OF STOREYS



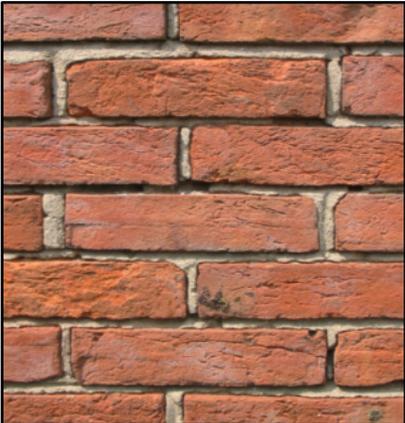
... very similar trend in terms of construction age and number of storeys

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

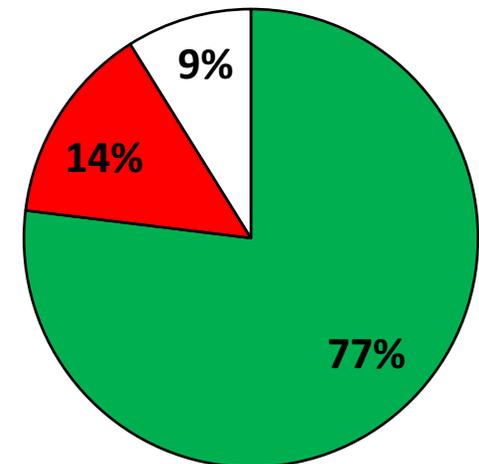
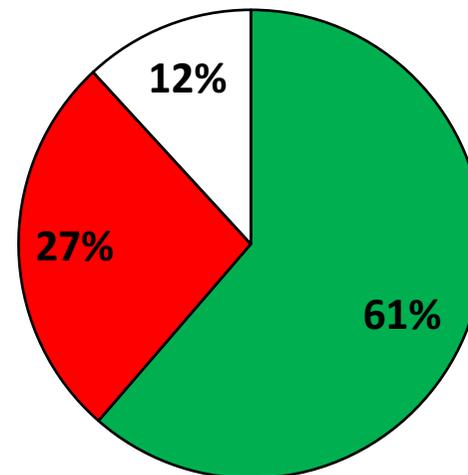
II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: How much set of buildings are comparable?

❖ MASONRY QUALITY

GOOD QUALITY MASONRY	BAD QUALITY MASONRY
	
UNKNOWN MASONRY QUALITY	

LG Buildings. Vs. UG Buildings:



..... according to macro parameters the set of data (UG and LG) are certainly comparable in terms of vulnerability (against inertial forces)

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

DETECTED DAMAGE GRADE (LEVEL AND EXTENT)

structural and non structural members
according to AeDES form filled by team of
experts in the post-earthquake emergency

Damage level - extension Structural component Pre-existing damage	DAMAGE ⁽¹⁾									Null
	D4-D5 Very Heavy			D2-D3 Medium-Severe			D1 Light			
	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	> 2/3	1/3 - 2/3	< 1/3	
1 Vertical structures	A	B	C	D	E	F	G	H	I	L
2 Floors	<input type="checkbox"/>									
3 Stairs	<input type="checkbox"/>									
4 Roof	<input type="checkbox"/>									
5 Infills and partitions	<input type="checkbox"/>									

DAMAGE D0



DAMAGE D1



DAMAGE D2-D3



DAMAGE D4-D5



NULL

LIGHT

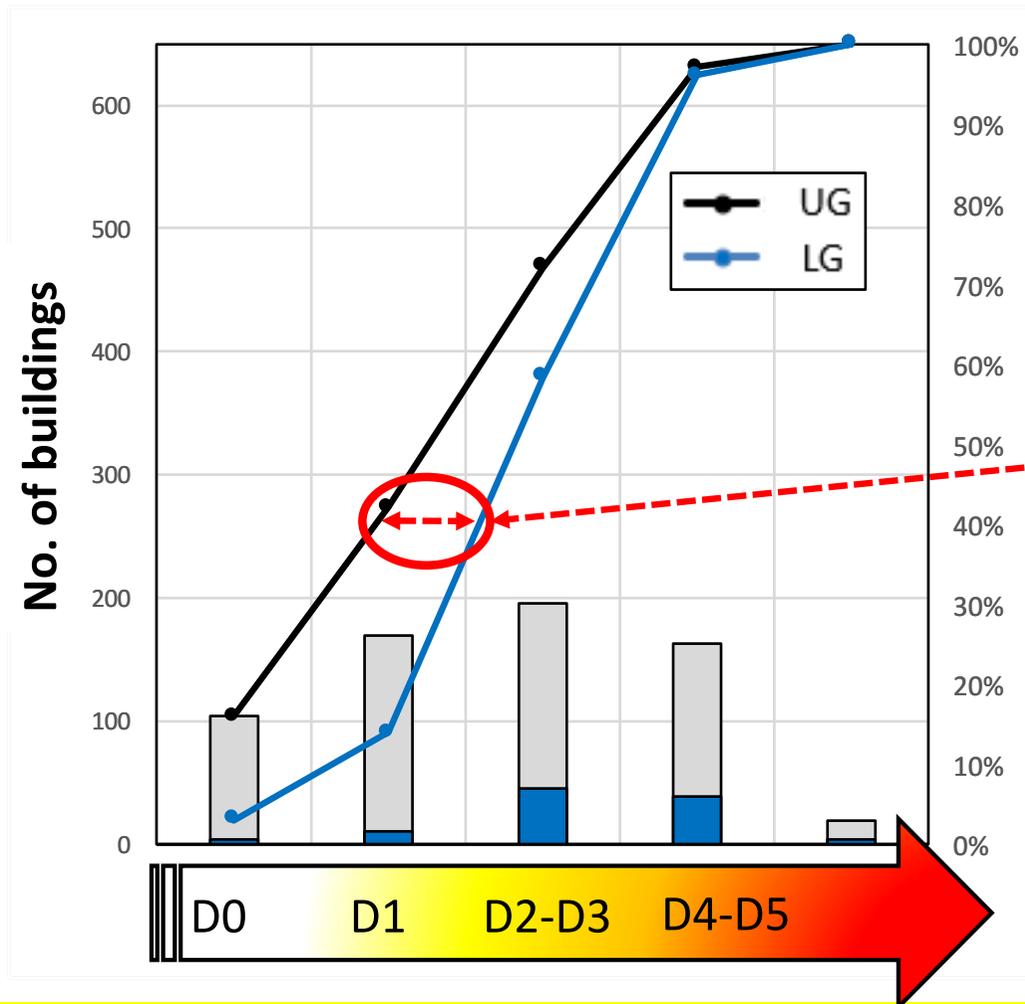
MEDIUM-SEVERE

VERY HEAVY

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: **Damage level?**



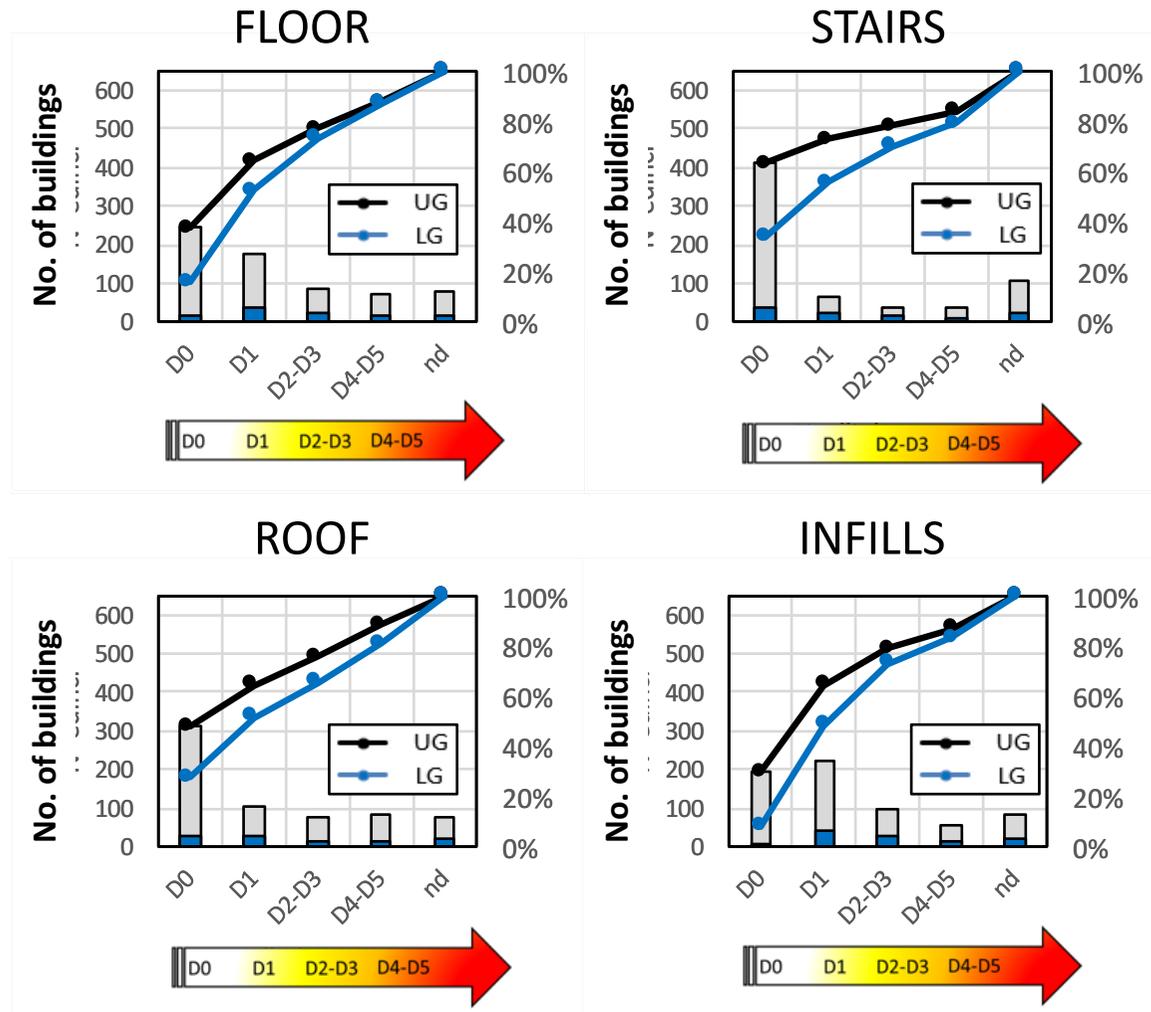
Cumulative trend
Buildings subjected to liquefaction (LG) show a higher average damage than that detected on buildings not affected by liquefaction (UG)

- ...liquefaction provides different types of damage but they are severe;
- ...liquefaction damage may be added to seismic damage due to inertial forces

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: **Damage level?**

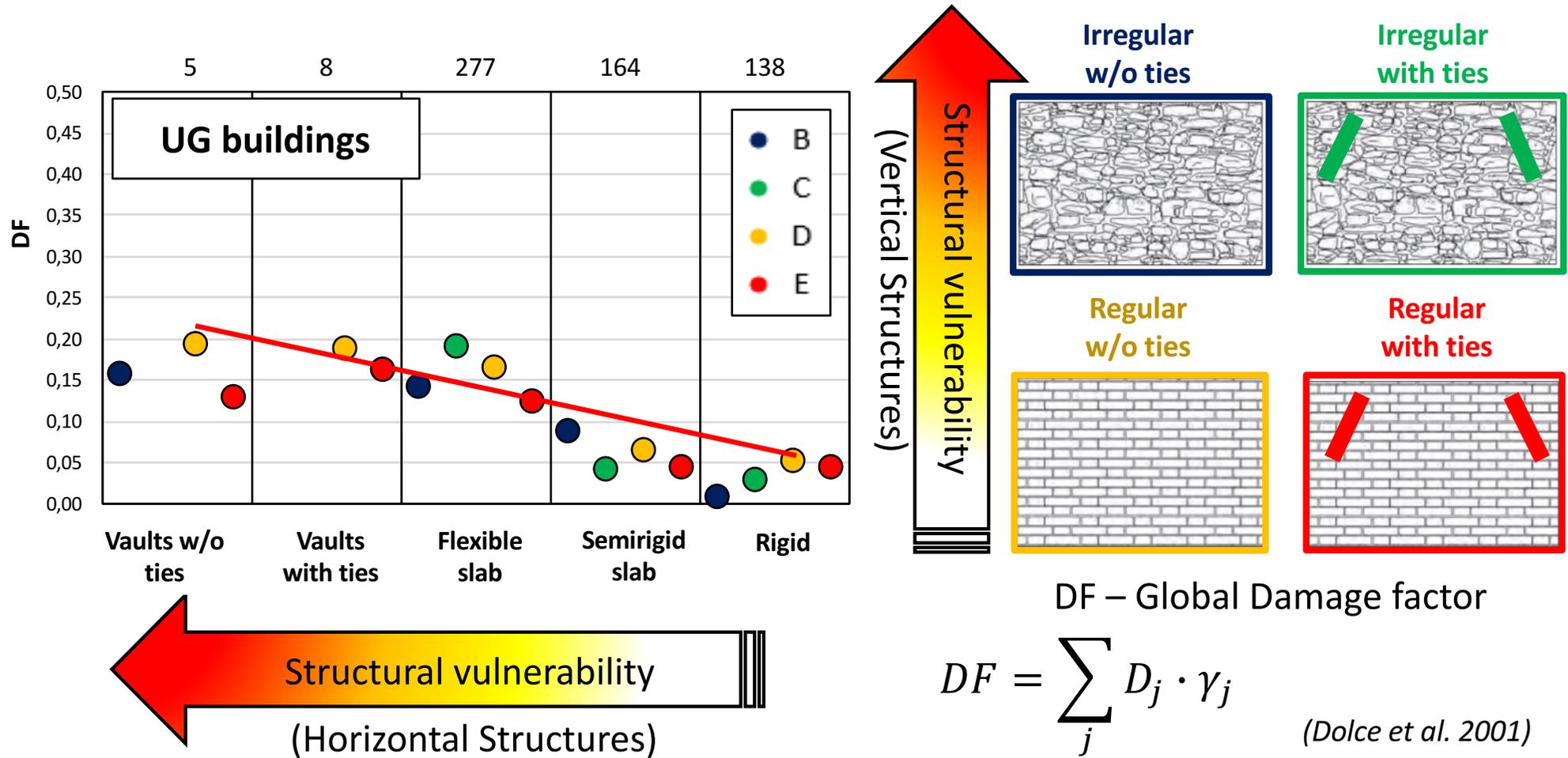


- ...a similar trend has been found for all structural and non structural components
- ...max difference observed on stairs (commonly slightly damaged due to inertial forces)

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

LG Buildings. Vs. UG Buildings: Vulnerability influence?



➤ ...seismic vulnerability clearly affected the empirical damage on UG set of buildings

DAMAGE ANALYSIS – EMPIRICAL DAMAGE

II. COMPARATIVE ANALYSIS OF EMPIRICAL DAMAGE

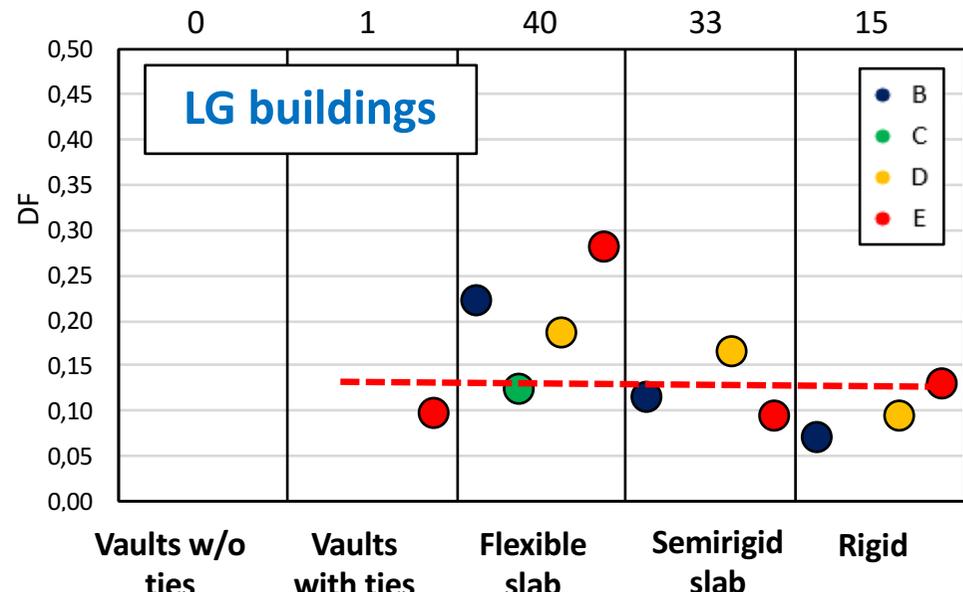
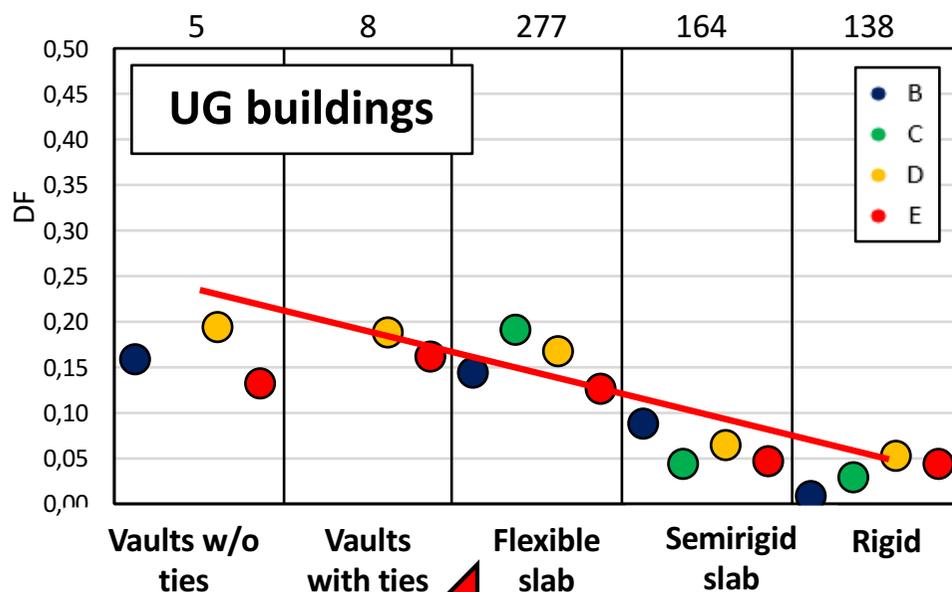
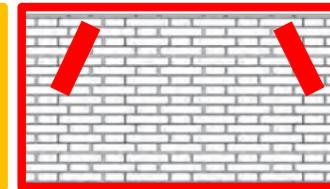
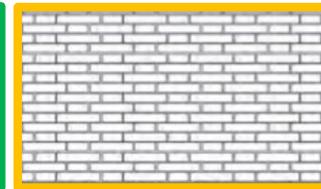
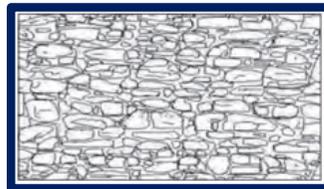
LG Buildings. Vs. UG Buildings: Vulnerability influence?

Irregular
w/o ties

Irregular
with ties

Regular
w/o ties

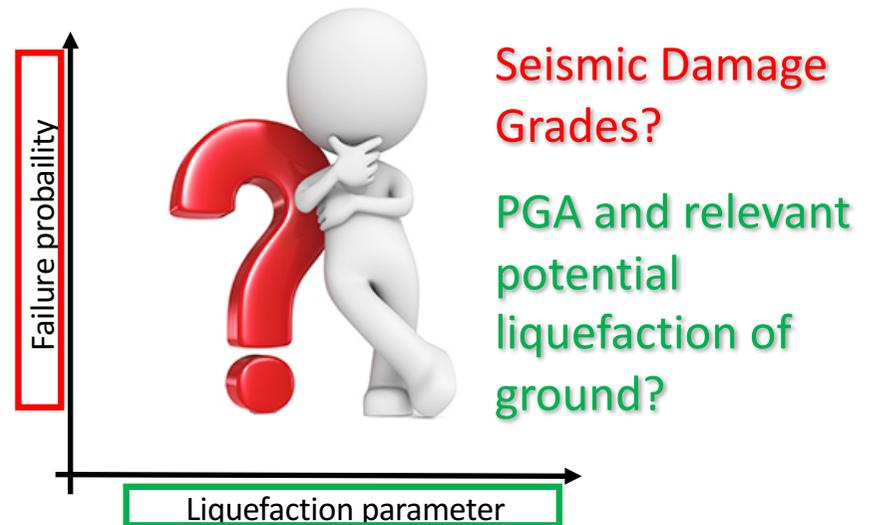
Regular
with ties



➤ ...sesimic vulnerability clearly affected the empirical damage on UG set of buildings while it seems not correlated to the empirical damage of LG buildings

OBJECTIVES

III. LIQUEFACTION FRAGILITY CURVES:
Which parameters should be used and what is the fragility curve trend?

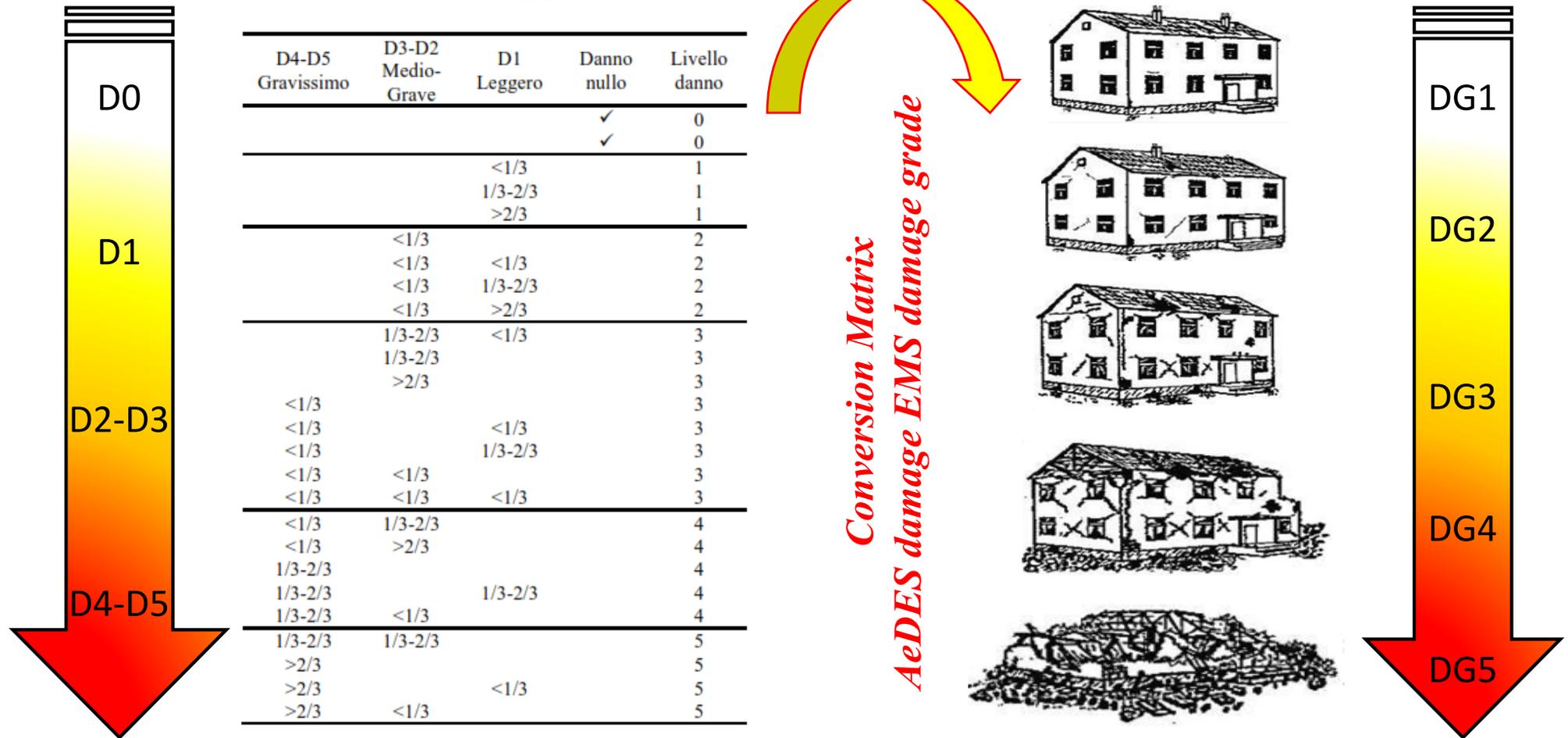


DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

Seismic Damage Grades: Defined according to the European Macroseismic scale EMS-98

The AeDES form (Baggio et al. 2008)



Dolce et al. 2017

➤ ...for each building it is possible to associate a damage grade from the empirical damage

DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

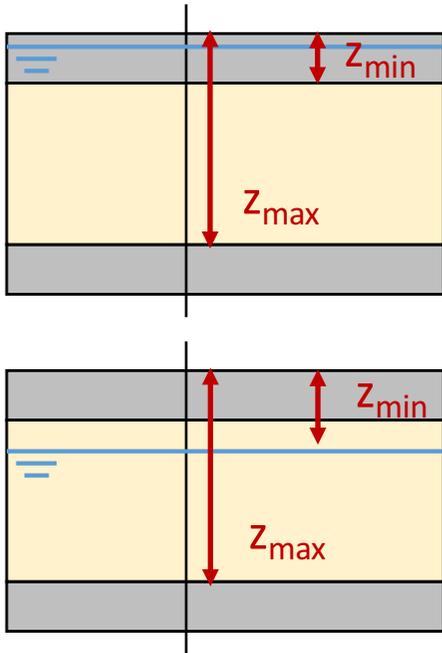
PGA and relevant potential liquefaction of ground?

Iwasaki et al., 1984

$$I_{AM} = \frac{1}{1 + z_{min}} \int_{z_{min}}^{z_{max}} r_u^2 dz$$

$$r_u = \frac{\Delta u}{\sigma'_{v0}} = f\left(F_s = \frac{CRR}{CSR}\right) \quad \begin{array}{l} \text{se } F_s \leq 1 \rightarrow r_u = 1 \\ \text{se } F_s > 1 \rightarrow r_u = (F_s)^{1/b} \end{array}$$

The exponent b is a function of the cyclic resistance of the soil (defined in laboratory tests)



- It has a clear physical meaning (**depends on the most superficial liquefiable material layer and its equivalent thickness**)
- It takes into account the increase in interstitial pressure even in the layers in which the liquefaction is not carried out ($r_u < 1$)
- Directly correlated with the post-cyclic consolidation settlement in free-field conditions (as a consequence of the structural damage)
- **It can be easily determined by simplified assessment of susceptibility to liquefaction** (estimation of r_u as a function of a safety factor, F_s) and from the results of dynamic analyzes in effective tensions (rigorous calculation of r_u).
- Finally, assuming the liquefaction for the entire potentially liquefiable layer, the index can be calculated **on the basis of stratigraphic knowledge**

DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

PGA and relevant potential liquefaction of ground?

$$I_{AM} = \frac{1}{1 + z_{min}} \int_{z_{min}}^{z_{max}} r_u^2 dz$$

It has been assumed

$$r_u = 1$$

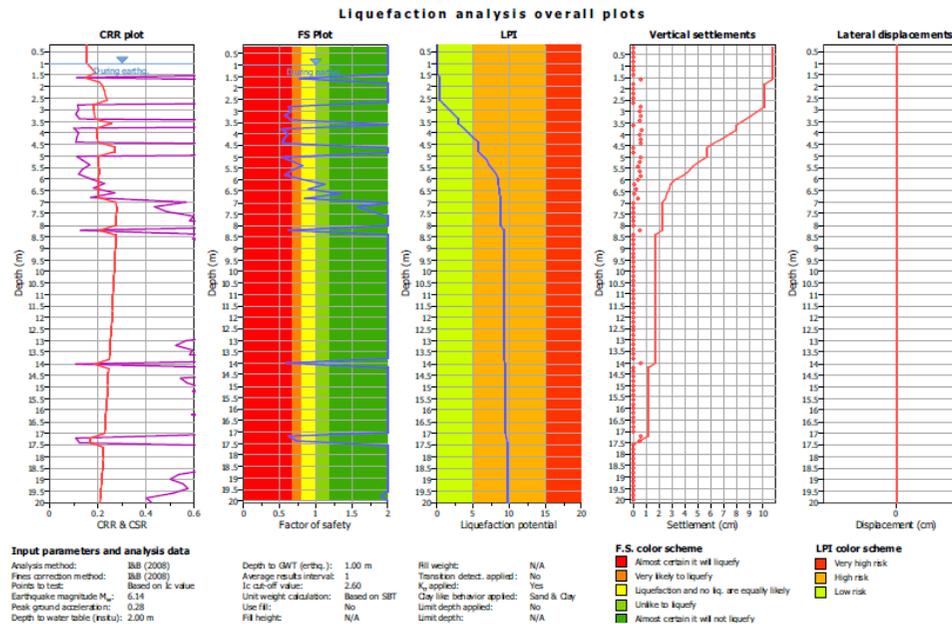
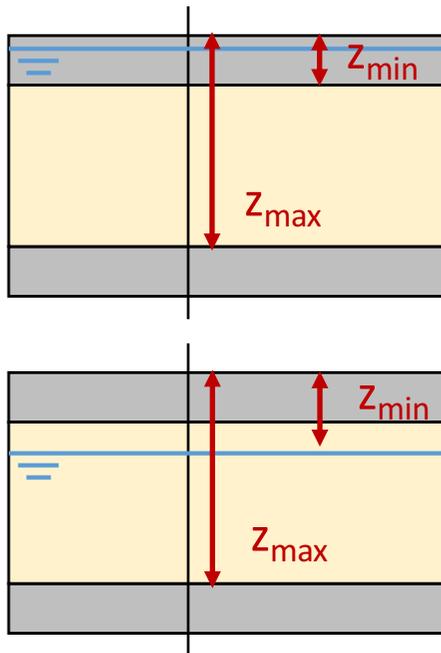
$$I_{AM} = \frac{z_{max} - z_{min}}{1 + z_{min}}$$

$$r_u = \frac{\Delta u}{\sigma'_{v0}} = f\left(F_s = \frac{CRR}{CSR}\right)$$

Iwasaki et al., 1984

se $F_s \leq 1 \rightarrow r_u = 1$

se $F_s > 1 \rightarrow r_u = (F_s)^{1/b}$



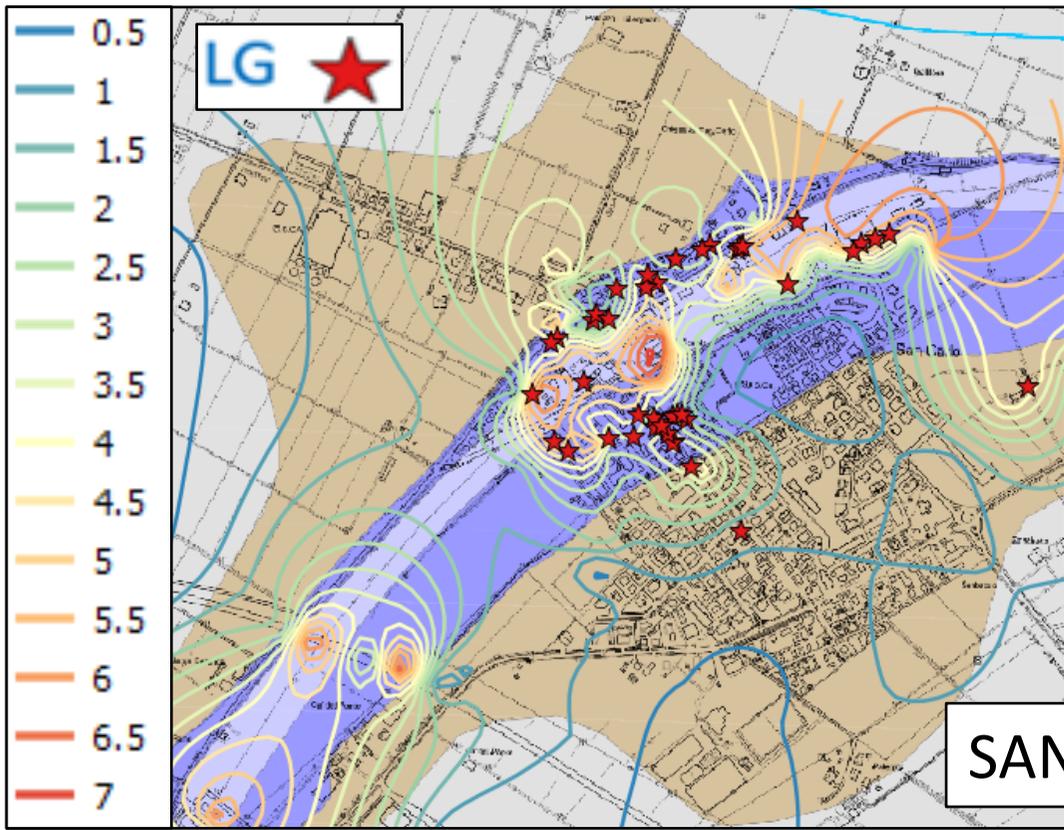
The exponent b is a function of the cyclic resistance of the soil (defined in laboratory tests)

DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

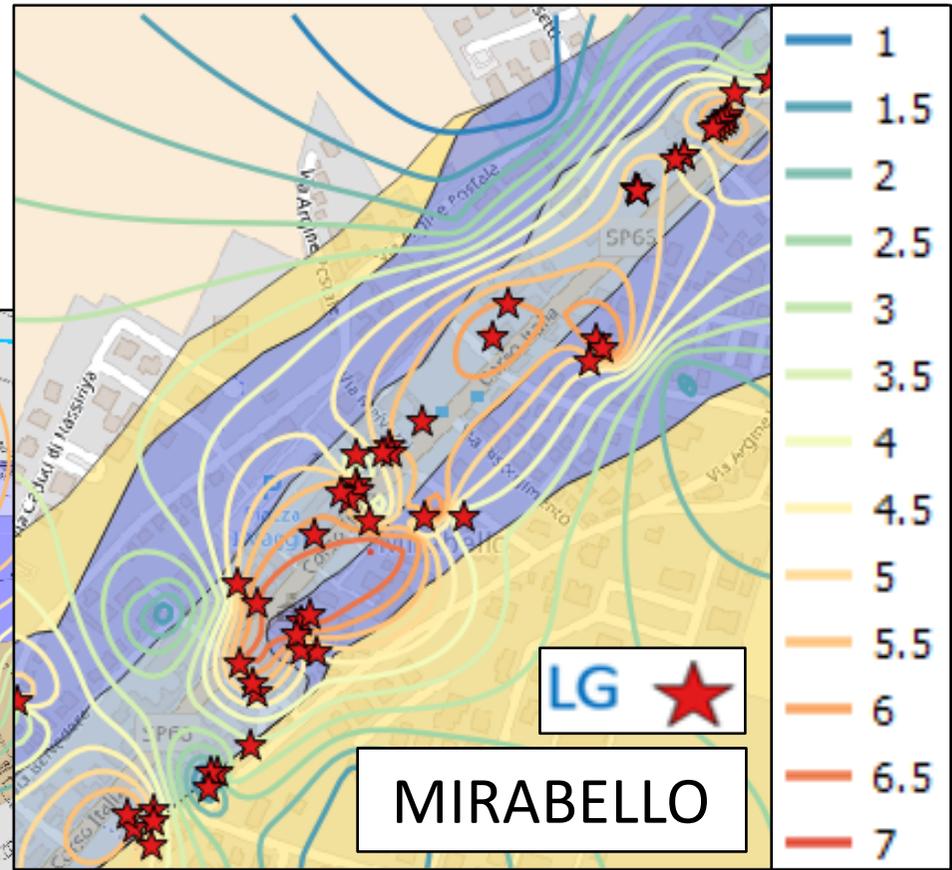
Δz MAP OF LAYER POTENTIAL LIQUEFACT

Δz [m]

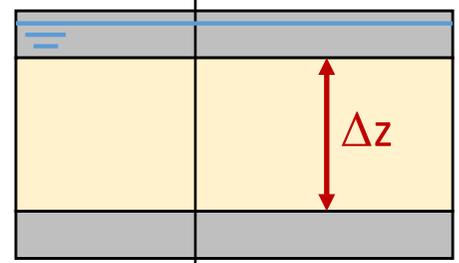


SAN CARLO

Δz [m]



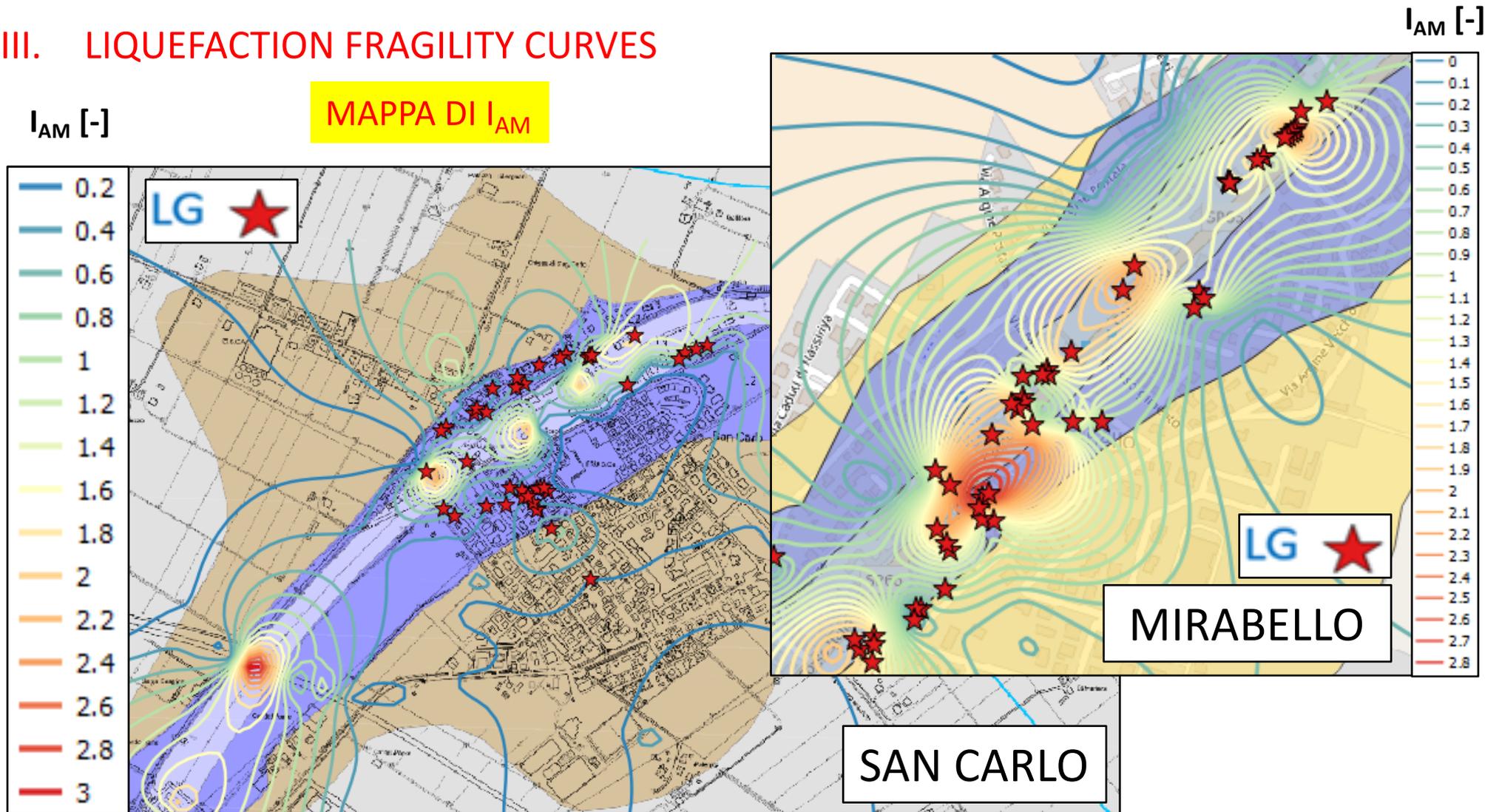
MIRABELLO



The spatial distribution of the thickness of potentially liquefiable material appears to be well correlated with the damage to the structures induced by liquefaction

DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

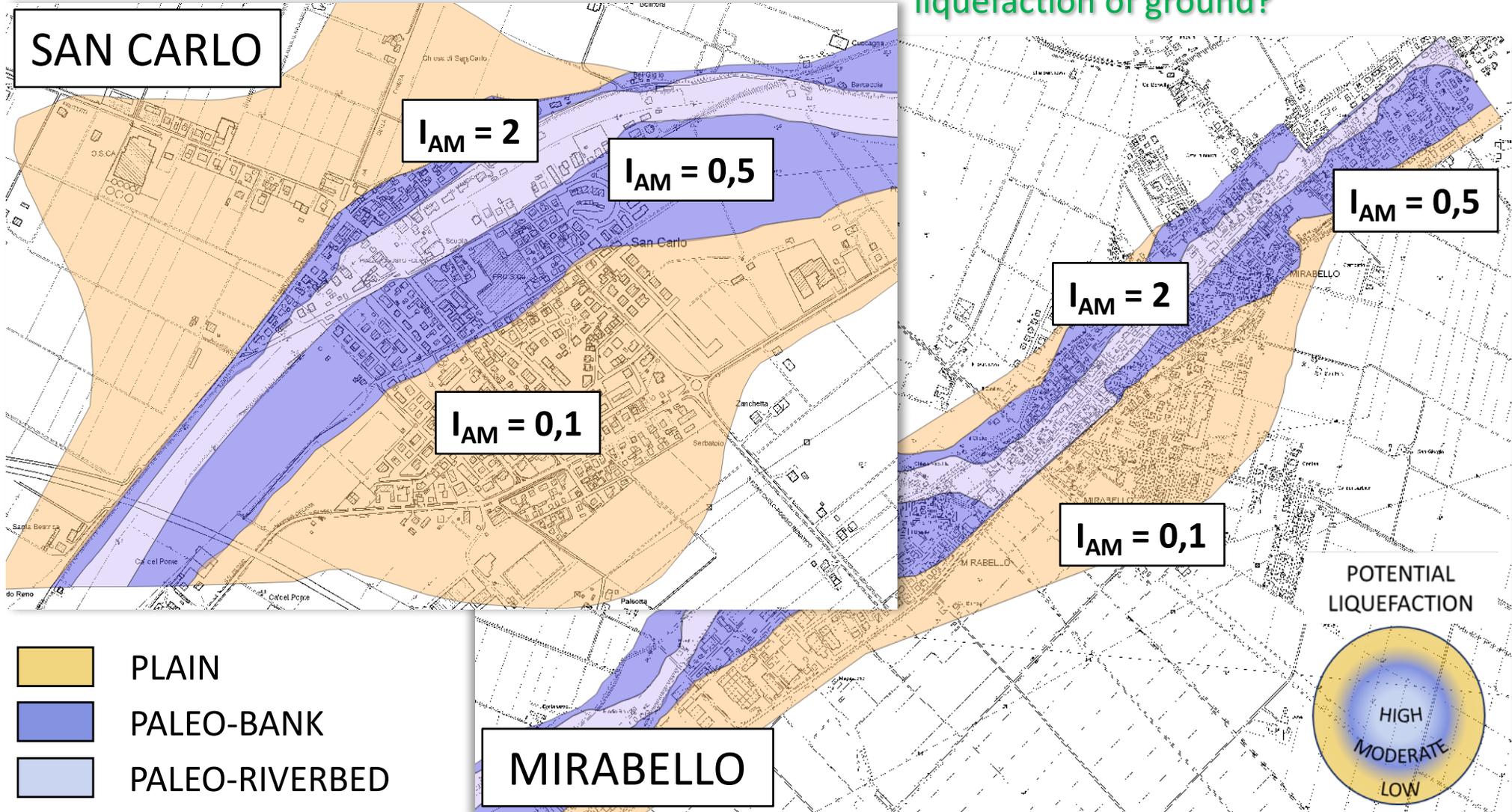


... I_{AM} is well correlated to the liquefaction induced damage

DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

PGA and relevant potential liquefaction of ground?



DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

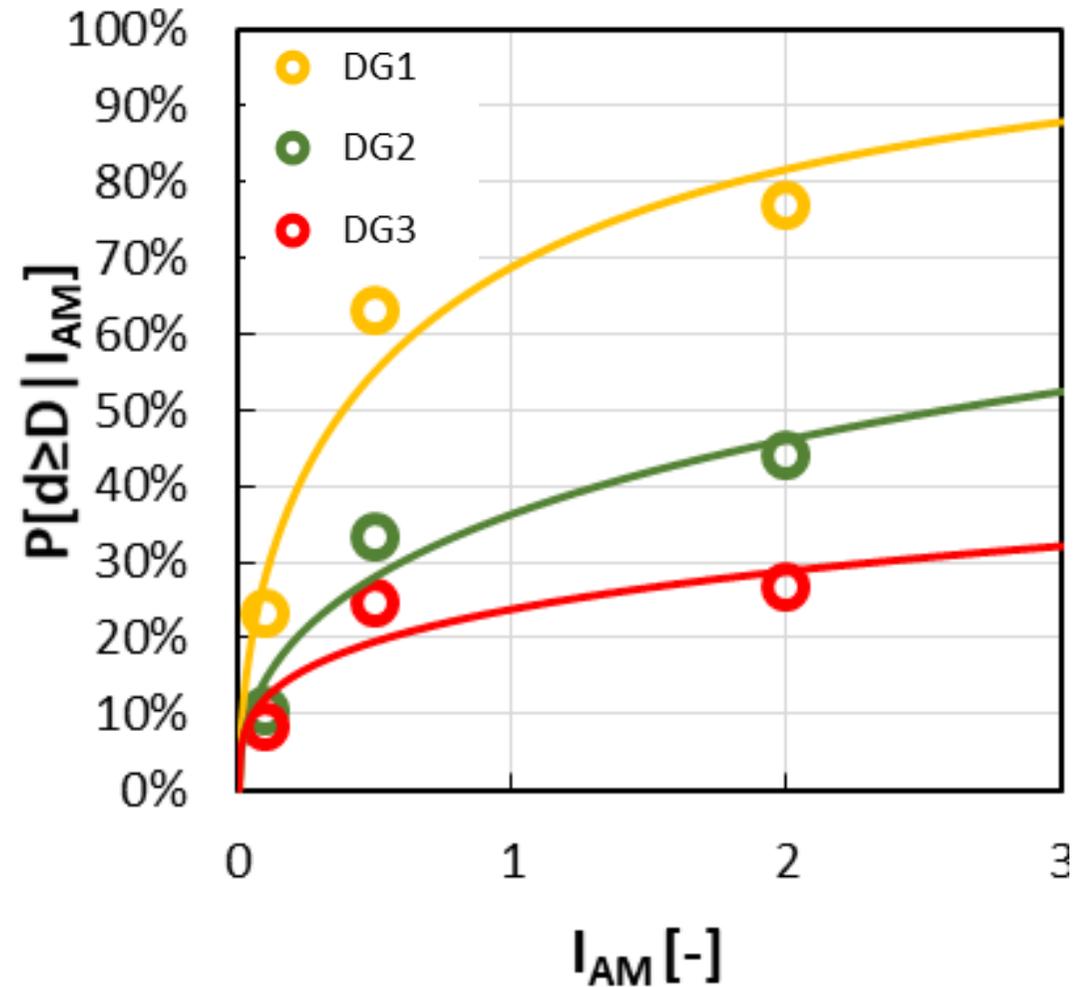
MODELLO DI PREVISIONE ESPONENZIALE
(Amiri et al. 2007;
Rossetto e Elnashai 2003;)

$$P[DG \geq dg | I_{AM}] = 1 - e^{-\alpha I_{AM}^{\beta}}$$

EXPONENTIAL PREVISION MODEL

nonlinear Least Squares
Estimation (LSE) fitting

— LSE DG1
— LSE DG2
— LSE DG3



- Insufficient data for DG4 and DG5 curves; more I_{AM} values needed
- Damage grades are based on seismic convention
- These curves are related to a given PGA range and it has been assumed $r_u = 1$

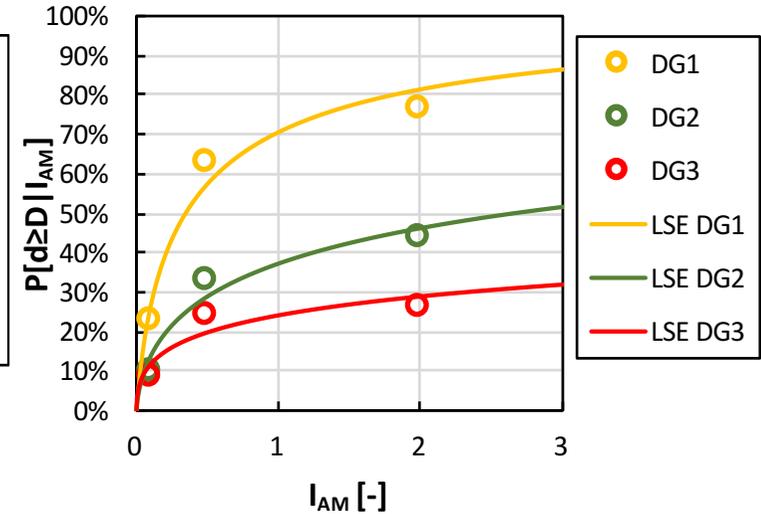
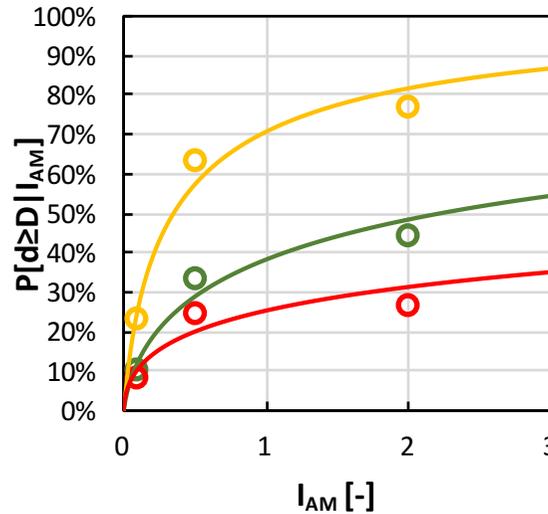
DAMAGE ANALYSIS – PROBABILITY OF DAMAGE

III. LIQUEFACTION FRAGILITY CURVES

LOGNORMAL PREVISION MODEL

$$\Phi\left(\frac{\ln(x) - \mu}{\sigma}\right)$$

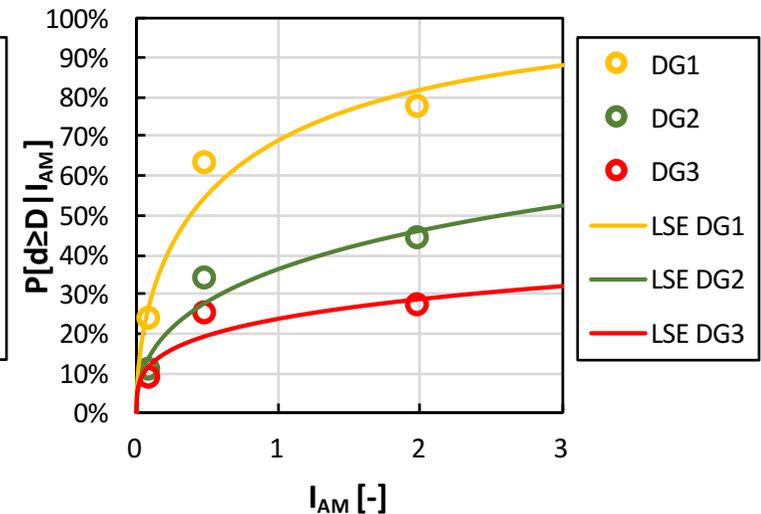
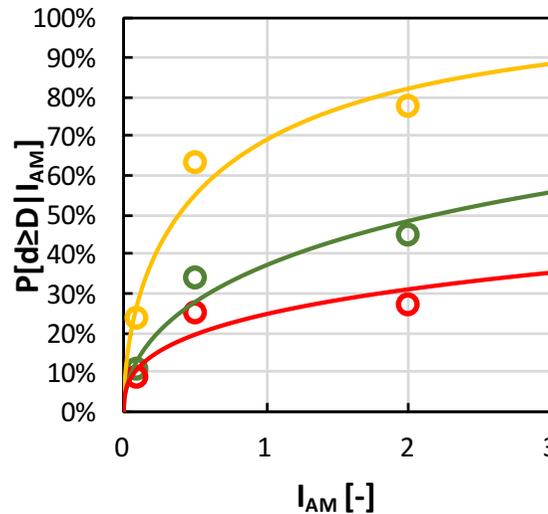
Shinozuka et al, 2000;
Rota et al, 2008



EXPONENTIAL PREVISION MODEL

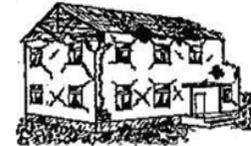
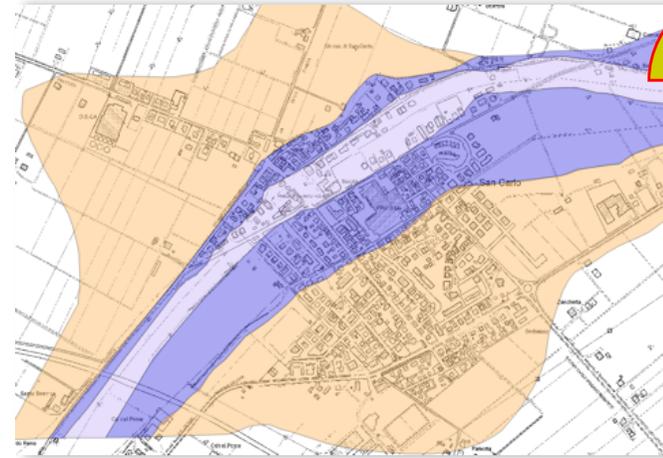
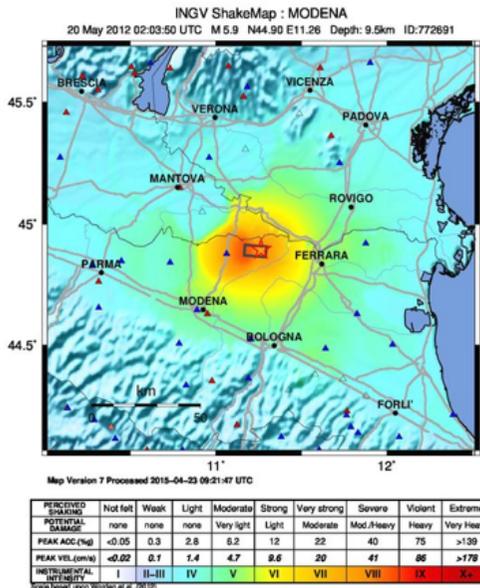
$$1 - \exp(-\alpha x^\beta)$$

Rossetto e Elnashai, 2003;
Amiri et al, 2007



- Insufficient data for DG3 and DG4 curves; more I_{AM} values needed
- Damage grades are based on seismic convention
- These curves are related to a given PGA range and it has been assumed ru = 1

FUTURE DEVELOPMENTS



DG1

DG2

DG3

DG4

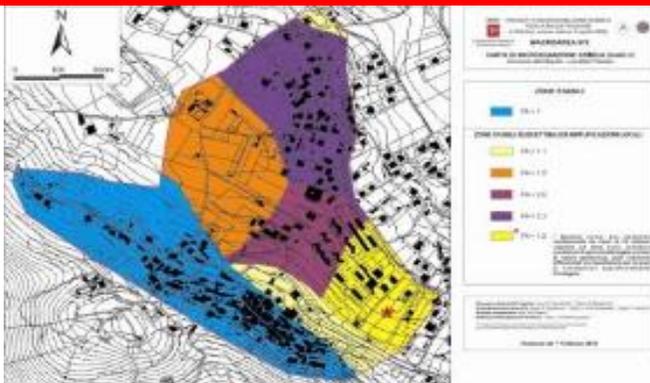
DG5

➤ PGA Scenario

➤different I_{AM} values

➤ I_{AM} Scenario

✓ Seismic Micronazionation



✓ Actual repair costs



➤damage grades accounting for liquefaction induced damage
➤ (amplification factor?)

Losses Scenario